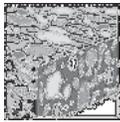


HSL-GR stainless steel, heavy duty anchor

| Anchor version | Benefits |
|---|---|
|  <p>HSL-GR</p> | <ul style="list-style-type: none"> - suitable for non-cracked C 20/25 to C 50/60 - high loading capacity - force-controlled expansion - reliable pull-down of the part fastened - no rotation in hole when tightening bolt |



Concrete

Basic loading data (for a single anchor)

All data in this section applies to

- Correct setting (See setting instruction)
- No edge distance and spacing influence
- Concrete as specified in the table
- **Steel** failure
- Minimum base material thickness
- Concrete C 20/25, $f_{ck,cube} = 25 \text{ N/mm}^2$

For details see Simplified design method

Mean ultimate resistance

| Anchor size | Hilti technical data for non-cracked concrete | | | | |
|-------------------------|---|------|------|------|-------|
| | M8 | M10 | M12 | M16 | M20 |
| Tensile $N_{Ru,m}$ [kN] | 26,9 | 39,2 | 47,9 | 66,9 | 93,5 |
| Shear $V_{Ru,m}$ [kN] | 26,3 | 42,0 | 57,8 | 84,0 | 115,5 |

Characteristic resistance

| Anchor size | Hilti technical data for non-cracked concrete | | | | |
|-----------------------|---|------|------|------|-------|
| | M8 | M10 | M12 | M16 | M20 |
| Tensile N_{Rk} [kN] | 23,4 | 29,5 | 36,1 | 50,4 | 70,4 |
| Shear V_{Rk} [kN] | 25,0 | 40,0 | 55,0 | 80,0 | 110,0 |

Design resistance

| Anchor size | Hilti technical data for non-cracked concrete | | | | |
|-----------------------|---|------|------|------|------|
| | M8 | M10 | M12 | M16 | M20 |
| Tensile N_{Rd} [kN] | 13,0 | 16,4 | 20,1 | 28,1 | 39,2 |
| Shear V_{Rd} [kN] | 16,0 | 25,6 | 35,3 | 51,3 | 70,5 |

Recommended loads ^{a)}

| | | Hilti technical data for non-cracked concrete | | | | |
|-------------------|------|---|------|------|------|------|
| Anchor size | | M8 | M10 | M12 | M16 | M20 |
| Tensile N_{rec} | [kN] | 9,3 | 11,7 | 14,3 | 20,0 | 28,0 |
| Shear V_{rec} | [kN] | 11,4 | 18,3 | 25,2 | 36,6 | 50,4 |

a) With overall partial safety factor for action $\gamma = 1,4$. The partial safety factors for action depend on the type of loading and shall be taken from national regulations.

Materials

Mechanical properties of HSL-GR

| Anchor size | M8 | M10 | M12 | M16 | M20 |
|---|------|------|-------|-------|-------|
| Nominal tensile strength f_{uk} | 700 | 700 | 700 | 700 | 700 |
| Yield strength f_{yk} | 450 | 450 | 450 | 450 | 450 |
| Stressed cross-section A_s | 36,6 | 58,0 | 84,3 | 157 | 245 |
| Moment of resistance W | 31,2 | 62,3 | 109,2 | 277,5 | 540,9 |
| Design bending resistance without sleeve $M_{Rd,s}$ | 16,8 | 33,5 | 58,8 | 149,4 | 291,3 |

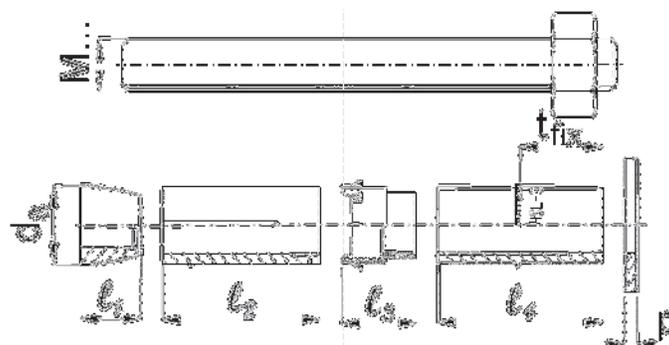
Material quality

| Part | Material |
|--------------------|----------------|
| Bolt, threaded rod | steel grade A4 |

Anchor dimensions

Dimensions of HSL-GR

| Thread size | t_{fix} [mm] | | d_s [mm] | l_1 [mm] | l_2 [mm] | l_3 [mm] | l_4 [mm] | | p [mm] |
|-------------|----------------|-----|------------|------------|------------|------------|------------|-----|----------|
| | min | max | | | | | min | max | |
| M8 | 5 | 200 | 11,8 | 8,5 | 26 | 15,2 | 26 | 221 | 3 |
| M10 | 5 | 200 | 14,8 | 10,8 | 30 | 17,2 | 29 | 224 | 4 |
| M12 | 5 | 200 | 17,6 | 12 | 32 | 20 | 32 | 227 | 5 |
| M16 | 10 | 200 | 23,6 | 18 | 46 | 24,4 | 43 | 233 | 5 |
| M20 | 10 | 200 | 27,6 | 22 | 57 | 31,5 | 51 | 241 | 6 |

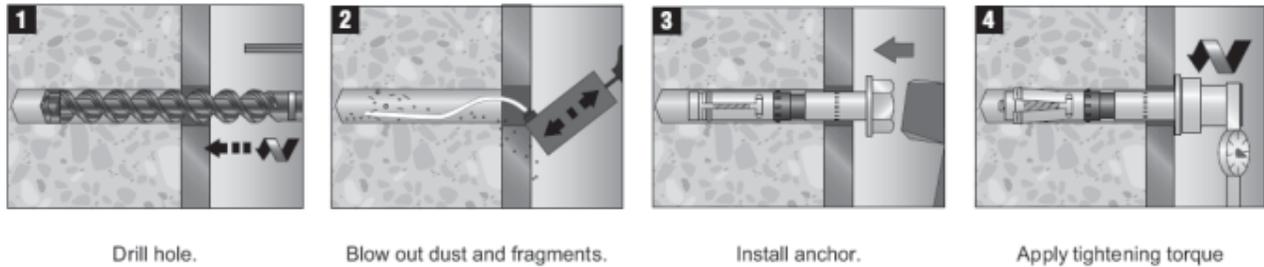


Setting

installation equipment

| Anchor size | M8 | M10 | M12 | M16 | M20 |
|---------------|--------------------------------------|-----|-----|-------------|-----|
| Rotary hammer | TE2 – TE16 | | | TE40 – TE70 | |
| Other tools | hammer, torque wrench, blow out pump | | | | |

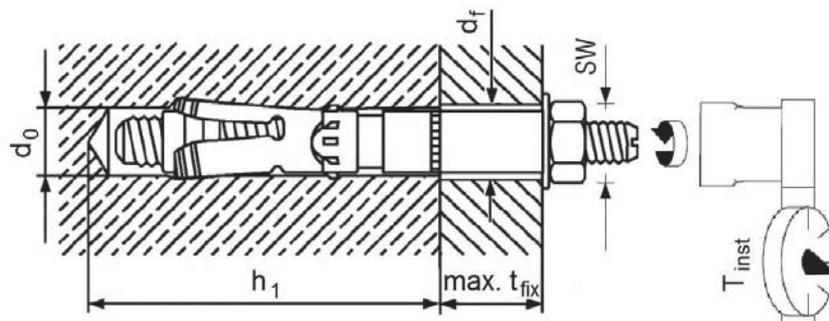
Setting instruction



For detailed information on installation see instruction for use given with the package of the product.

For technical data for anchors in diamond drilled holes please contact the Hilti Technical advisory service.

Setting details: depth of drill hole h_1 and effective anchorage depth h_{ef}

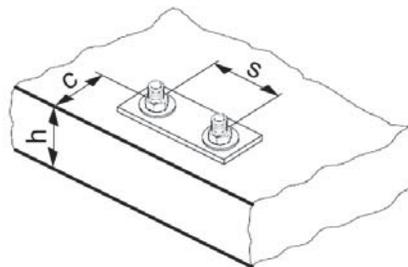


Setting details

| Anchor size | M8 | M10 | M12 | M16 | M20 | |
|---|---------------------|------|------|------|-------|-------|
| Nominal diameter of drill bit | d_0 [mm] | 12 | 15 | 18 | 24 | 28 |
| Cutting diameter of drill bit | $d_{cut} \leq$ [mm] | 12,5 | 15,5 | 18,5 | 24,55 | 28,55 |
| Depth of drill hole | $h_1 \geq$ [mm] | 80 | 90 | 105 | 125 | 155 |
| Diameter of clearance hole in the fixture | $d_f \leq$ [mm] | 14 | 17 | 20 | 26 | 31 |
| Effective anchorage depth | h_{ef} [mm] | 60 | 70 | 80 | 100 | 125 |
| Torque moment | T_{inst} [Nm] | 20 | 35 | 60 | 80 | 160 |
| Width across | SW [mm] | 13 | 17 | 19 | 24 | 30 |

Setting parameters

| Anchor size | | | M8 | M10 | M12 | M16 | M20 |
|--|-------------|------|-----|-----|-----|-----|-------|
| Minimum base material thickness | h_{min} | [mm] | 120 | 140 | 160 | 200 | 250 |
| Minimum spacing | s_{min} | [mm] | 100 | 160 | 240 | 240 | 300 |
| Minimum edge distance | c_{min} | [mm] | 60 | 70 | 80 | 100 | 150 |
| Critical spacing for splitting failure | $s_{cr,sp}$ | [mm] | 270 | 300 | 330 | 380 | 480 |
| Critical edge distance for splitting failure | $c_{cr,sp}$ | [mm] | 135 | 150 | 165 | 190 | 240 |
| Critical spacing for concrete cone failure | $s_{cr,N}$ | [mm] | 180 | 210 | 240 | 300 | 375 |
| Critical edge distance for concrete cone failure | $c_{cr,N}$ | [mm] | 90 | 105 | 120 | 150 | 187,5 |



For spacing (edge distance) smaller than critical spacing (critical edge distance) the design loads have to be reduced.

Critical spacing and critical edge distance for splitting failure apply only for non-cracked concrete. For cracked concrete only the critical spacing and critical edge distance for concrete cone failure are decisive.

Simplified design method

Simplified version of the design method according ETAG 001, Annex C.

- Influence of concrete strength
- Influence of edge distance
- Influence of spacing
- Valid for a group of two anchors. (The method may also be applied for anchor groups with more than two anchors or more than one edge. The influencing factors must then be considered for each edge distance and spacing. The calculated design loads are then on the save side: They will be lower than the exact values according ETAG 001, Annex C.)

The design method is based on the following simplification:

- No different loads are acting on individual anchors (no eccentricity)

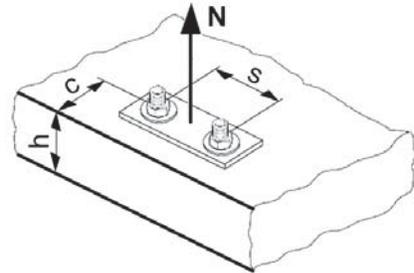
The values are valid for one anchor.

For more complex fastening applications please use the anchor design software PROFIS Anchor.

Tension loading

The design tensile resistance is the lower value of

- Steel resistance: $N_{Rd,s}$
- Concrete pull-out resistance: $N_{Rd,p} = N_{Rd,p}^0 \cdot f_B$
- Concrete cone resistance: $N_{Rd,c} = N_{Rd,c}^0 \cdot f_B \cdot f_{1,N} \cdot f_{2,N} \cdot f_{3,N} \cdot f_{re,N}$
- Concrete splitting resistance (only non-cracked concrete):
 $N_{Rd,sp} = N_{Rd,c}^0 \cdot f_B \cdot f_{1,sp} \cdot f_{2,sp} \cdot f_{3,sp} \cdot f_{h,sp} \cdot f_{re,N}$



Basic design tensile resistance

Design steel resistance $N_{Rd,s}$

| Anchor size | M8 | M10 | M12 | M16 | M20 |
|-----------------|------|------|------|------|------|
| $N_{Rd,s}$ [kN] | 13,7 | 21,7 | 31,6 | 58,8 | 91,7 |

Design pull-out resistance $N_{Rd,p} = N_{Rd,p}^0 \cdot f_B$

| Anchor size | M8 | M10 | M12 | M16 | M20 |
|-------------------|---------------------|-----|-----|-----|-----|
| $N_{Rd,p}^0$ [kN] | No pull-out failure | | | | |

Design concrete cone resistance $N_{Rd,c} = N_{Rd,c}^0 \cdot f_B \cdot f_{1,N} \cdot f_{2,N} \cdot f_{3,N} \cdot f_{re,N}$

Design splitting resistance $N_{Rd,sp} = N_{Rd,c}^0 \cdot f_B \cdot f_{1,sp} \cdot f_{2,sp} \cdot f_{3,sp} \cdot f_{h,sp} \cdot f_{re,N}$

| Anchor size | M8 | M10 | M12 | M16 | M20 |
|-------------------|------|------|------|------|------|
| $N_{Rd,c}^0$ [kN] | 13,0 | 16,4 | 20,1 | 28,1 | 39,2 |

Influencing factors

Influence of concrete strength

| Concrete strength designation (ENV 206) | C 20/25 | C 25/30 | C 30/37 | C 35/45 | C 40/50 | C 45/55 | C 50/60 |
|---|---------|---------|---------|---------|---------|---------|---------|
| $f_B = (f_{ck,cube}/25N/mm^2)^{1/2}$ a) | 1 | 1,1 | 1,22 | 1,34 | 1,41 | 1,48 | 1,55 |

a) $f_{ck,cube}$ = concrete compressive strength, measured on cubes with 150 mm side length

Influence of edge distance a)

| $c/c_{cr,N}$ | 0,1 | 0,2 | 0,3 | 0,4 | 0,5 | 0,6 | 0,7 | 0,8 | 0,9 | 1 |
|---|------|------|------|------|------|------|------|------|------|---|
| $c/c_{cr,sp}$ | | | | | | | | | | |
| $f_{1,N} = 0,7 + 0,3 \cdot c/c_{cr,N} \leq 1$ | 0,73 | 0,76 | 0,79 | 0,82 | 0,85 | 0,88 | 0,91 | 0,94 | 0,97 | 1 |
| $f_{1,sp} = 0,7 + 0,3 \cdot c/c_{cr,sp} \leq 1$ | | | | | | | | | | |
| $f_{2,N} = 0,5 \cdot (1 + c/c_{cr,N}) \leq 1$ | 0,55 | 0,60 | 0,65 | 0,70 | 0,75 | 0,80 | 0,85 | 0,90 | 0,95 | 1 |
| $f_{2,sp} = 0,5 \cdot (1 + c/c_{cr,sp}) \leq 1$ | | | | | | | | | | |

a) The edge distance shall not be smaller than the minimum edge distance c_{min} given in the table with the setting details. These influencing factors must be considered for every edge distance.

Influence of anchor spacing ^{a)}

| $s/s_{cr,N}$ | 0,1 | 0,2 | 0,3 | 0,4 | 0,5 | 0,6 | 0,7 | 0,8 | 0,9 | 1 |
|---|------|------|------|------|------|------|------|------|------|---|
| $s/s_{cr,sp}$ | | | | | | | | | | |
| $f_{3,N} = 0,5 \cdot (1 + s/s_{cr,N}) \leq 1$ | 0,55 | 0,60 | 0,65 | 0,70 | 0,75 | 0,80 | 0,85 | 0,90 | 0,95 | 1 |
| $f_{3,sp} = 0,5 \cdot (1 + s/s_{cr,sp}) \leq 1$ | | | | | | | | | | |

a) The anchor spacing shall not be smaller than the minimum anchor spacing s_{min} given in the table with the setting details. This influencing factor must be considered for every anchor spacing.

Influence of base material thickness

| h/h_{ef} | 2,0 | 2,2 | 2,4 | 2,6 | 2,8 | 3,0 | 3,2 | 3,4 | 3,6 | $\geq 3,68$ |
|---|-----|------|------|------|------|------|------|------|------|-------------|
| $f_{h,sp} = [h/(2 \cdot h_{ef})]^{2/3}$ | 1 | 1,07 | 1,13 | 1,19 | 1,25 | 1,31 | 1,37 | 1,42 | 1,48 | 1,5 |

Influence of reinforcement

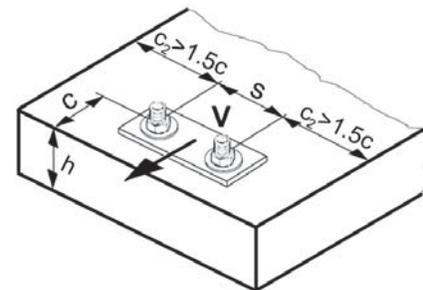
| Anchor size | M8 | M10 | M12 | M16 | M20 |
|---|-------------------|--------------------|-------------------|-----|-----|
| $f_{re,N} = 0,5 + h_{ef}/200\text{mm} \leq 1$ | 0,8 ^{a)} | 0,85 ^{a)} | 0,9 ^{a)} | 1 | 1 |

a) This factor applies only for dense reinforcement. If in the area of anchorage there is reinforcement with a spacing ≥ 150 mm (any diameter) or with a diameter ≤ 10 mm and a spacing ≥ 100 mm, then a factor $f_{re,N} = 1$ may be applied.

Shear loading

The design shear resistance is the lower value of

- Steel resistance: $V_{Rd,s}$
- Concrete pryout resistance: $V_{Rd,cp} = k \cdot N_{Rd,c}$
- Concrete edge resistance: $V_{Rd,c} = V_{Rd,c}^0 \cdot f_B \cdot f_h \cdot f_4 \cdot f_{hef} \cdot f_c$



Basic design shear resistance

Design steel resistance $V_{Rd,s}$

| Anchor size | M8 | M10 | M12 | M16 | M20 |
|-----------------|------|------|------|------|------|
| $V_{Rd,s}$ [kN] | 16,0 | 25,6 | 35,3 | 51,3 | 70,5 |

Design concrete pryout resistance $V_{Rd,cp} = k \cdot N_{Rd,c}$ ^{a)}

| Anchor size | M8 | M10 | M12 | M16 | M20 |
|-------------|-----|-----|-----|-----|-----|
| k | 1,8 | 2,0 | | | |

a) $N_{Rd,c}$: Design concrete cone resistance

Design concrete edge resistance ^{a)} $V_{Rd,c} = V_{Rd,c}^0 \cdot f_B \cdot f_h \cdot f_4 \cdot f_{hef} \cdot f_c$

| Anchor size | M8 | M10 | M12 | M16 | M20 |
|-------------------|------|------|------|------|------|
| $V_{Rd,c}^0$ [kN] | 11,4 | 16,5 | 22,4 | 36,2 | 46,9 |

a) For anchor groups only the anchors close to the edge must be considered.

Influencing factors

Influence of concrete strength

| Concrete strength designation (ENV 206) | C 20/25 | C 25/30 | C 30/37 | C 35/45 | C 40/50 | C 45/55 | C 50/60 |
|---|---------|---------|---------|---------|---------|---------|---------|
| $f_B = (f_{ck,cube}/25N/mm^2)^{1/2}$ a) | 1 | 1,1 | 1,22 | 1,34 | 1,41 | 1,48 | 1,55 |

a) $f_{ck,cube}$ = concrete compressive strength, measured on cubes with 150 mm side length

Influence of angle between load applied and the direction perpendicular to the free edge

| Angle β | 0° | 10° | 20° | 30° | 40° | 50° | 60° | 70° | 80° | ≥ 90° |
|---|----|------|------|------|------|------|------|------|------|-------|
| $f_\beta = \sqrt{\frac{1}{(\cos \alpha_v)^2 + \left(\frac{\sin \alpha_v}{2,5}\right)^2}}$ | 1 | 1,01 | 1,05 | 1,13 | 1,24 | 1,40 | 1,64 | 1,97 | 2,32 | 2,50 |

Influence of base material thickness

| h/c | 0,15 | 0,3 | 0,45 | 0,6 | 0,75 | 0,9 | 1,05 | 1,2 | 1,35 | ≥ 1,5 |
|--|------|------|------|------|------|------|------|------|------|-------|
| $f_h = \{h/(1,5 \cdot c)\}^{1/2} \leq 1$ | 0,32 | 0,45 | 0,55 | 0,63 | 0,71 | 0,77 | 0,84 | 0,89 | 0,95 | 1,00 |

Influence of anchor spacing and edge distance ^{a)} for concrete edge resistance: f_4

$$f_4 = (c/h_{ef})^{1,5} \cdot (1 + s / [3 \cdot c]) \cdot 0,5$$

| c/h _{ef} | Single anchor | Group of two anchors s/h _{ef} | | | | | | | | | | | | | | |
|-------------------|---------------|--|------|------|------|------|------|------|------|------|------|------|------|-------|-------|-------|
| | | 0,75 | 1,50 | 2,25 | 3,00 | 3,75 | 4,50 | 5,25 | 6,00 | 6,75 | 7,50 | 8,25 | 9,00 | 9,75 | 10,50 | 11,25 |
| 0,50 | 0,35 | 0,27 | 0,35 | 0,35 | 0,35 | 0,35 | 0,35 | 0,35 | 0,35 | 0,35 | 0,35 | 0,35 | 0,35 | 0,35 | 0,35 | 0,35 |
| 0,75 | 0,65 | 0,43 | 0,54 | 0,65 | 0,65 | 0,65 | 0,65 | 0,65 | 0,65 | 0,65 | 0,65 | 0,65 | 0,65 | 0,65 | 0,65 | 0,65 |
| 1,00 | 1,00 | 0,63 | 0,75 | 0,88 | 1,00 | 1,00 | 1,00 | 1,00 | 1,00 | 1,00 | 1,00 | 1,00 | 1,00 | 1,00 | 1,00 | 1,00 |
| 1,25 | 1,40 | 0,84 | 0,98 | 1,12 | 1,26 | 1,40 | 1,40 | 1,40 | 1,40 | 1,40 | 1,40 | 1,40 | 1,40 | 1,40 | 1,40 | 1,40 |
| 1,50 | 1,84 | 1,07 | 1,22 | 1,38 | 1,53 | 1,68 | 1,84 | 1,84 | 1,84 | 1,84 | 1,84 | 1,84 | 1,84 | 1,84 | 1,84 | 1,84 |
| 1,75 | 2,32 | 1,32 | 1,49 | 1,65 | 1,82 | 1,98 | 2,15 | 2,32 | 2,32 | 2,32 | 2,32 | 2,32 | 2,32 | 2,32 | 2,32 | 2,32 |
| 2,00 | 2,83 | 1,59 | 1,77 | 1,94 | 2,12 | 2,30 | 2,47 | 2,65 | 2,83 | 2,83 | 2,83 | 2,83 | 2,83 | 2,83 | 2,83 | 2,83 |
| 2,25 | 3,38 | 1,88 | 2,06 | 2,25 | 2,44 | 2,63 | 2,81 | 3,00 | 3,19 | 3,38 | 3,38 | 3,38 | 3,38 | 3,38 | 3,38 | 3,38 |
| 2,50 | 3,95 | 2,17 | 2,37 | 2,57 | 2,77 | 2,96 | 3,16 | 3,36 | 3,56 | 3,76 | 3,95 | 3,95 | 3,95 | 3,95 | 3,95 | 3,95 |
| 2,75 | 4,56 | 2,49 | 2,69 | 2,90 | 3,11 | 3,32 | 3,52 | 3,73 | 3,94 | 4,15 | 4,35 | 4,56 | 4,56 | 4,56 | 4,56 | 4,56 |
| 3,00 | 5,20 | 2,81 | 3,03 | 3,25 | 3,46 | 3,68 | 3,90 | 4,11 | 4,33 | 4,55 | 4,76 | 4,98 | 5,20 | 5,20 | 5,20 | 5,20 |
| 3,25 | 5,86 | 3,15 | 3,38 | 3,61 | 3,83 | 4,06 | 4,28 | 4,51 | 4,73 | 4,96 | 5,18 | 5,41 | 5,63 | 5,86 | 5,86 | 5,86 |
| 3,50 | 6,55 | 3,51 | 3,74 | 3,98 | 4,21 | 4,44 | 4,68 | 4,91 | 5,14 | 5,38 | 5,61 | 5,85 | 6,08 | 6,31 | 6,55 | 6,55 |
| 3,75 | 7,26 | 3,87 | 4,12 | 4,36 | 4,60 | 4,84 | 5,08 | 5,33 | 5,57 | 5,81 | 6,05 | 6,29 | 6,54 | 6,78 | 7,02 | 7,26 |
| 4,00 | 8,00 | 4,25 | 4,50 | 4,75 | 5,00 | 5,25 | 5,50 | 5,75 | 6,00 | 6,25 | 6,50 | 6,75 | 7,00 | 7,25 | 7,50 | 7,75 |
| 4,25 | 8,76 | 4,64 | 4,90 | 5,15 | 5,41 | 5,67 | 5,93 | 6,18 | 6,44 | 6,70 | 6,96 | 7,22 | 7,47 | 7,73 | 7,99 | 8,25 |
| 4,50 | 9,55 | 5,04 | 5,30 | 5,57 | 5,83 | 6,10 | 6,36 | 6,63 | 6,89 | 7,16 | 7,42 | 7,69 | 7,95 | 8,22 | 8,49 | 8,75 |
| 4,75 | 10,35 | 5,45 | 5,72 | 5,99 | 6,27 | 6,54 | 6,81 | 7,08 | 7,36 | 7,63 | 7,90 | 8,17 | 8,45 | 8,72 | 8,99 | 9,26 |
| 5,00 | 11,18 | 5,87 | 6,15 | 6,43 | 6,71 | 6,99 | 7,27 | 7,55 | 7,83 | 8,11 | 8,39 | 8,66 | 8,94 | 9,22 | 9,50 | 9,78 |
| 5,25 | 12,03 | 6,30 | 6,59 | 6,87 | 7,16 | 7,45 | 7,73 | 8,02 | 8,31 | 8,59 | 8,88 | 9,17 | 9,45 | 9,74 | 10,02 | 10,31 |
| 5,50 | 12,90 | 6,74 | 7,04 | 7,33 | 7,62 | 7,92 | 8,21 | 8,50 | 8,79 | 9,09 | 9,38 | 9,67 | 9,97 | 10,26 | 10,55 | 10,85 |

a) The anchor spacing and the edge distance shall not be smaller than the minimum anchor spacing s_{min} and the minimum edge distance c_{min} .

Influence of embedment depth

| Anchor size | M8 | M10 | M12 | M16 | M20 |
|--|------|------|------|------|------|
| $f_{hef} = 0,05 \cdot (h_{ef} / d)^{1,68}$ | 0,75 | 0,67 | 0,61 | 0,55 | 0,62 |

Influence of edge distance ^{a)}

| c/d | 4 | 6 | 8 | 10 | 15 | 20 | 30 | 40 |
|------------------------|------|------|------|------|------|------|------|------|
| $f_c = (d / c)^{0,19}$ | 0,77 | 0,71 | 0,67 | 0,65 | 0,60 | 0,57 | 0,52 | 0,50 |

a) The edge distance shall not be smaller than the minimum edge distance c_{min} .

Combined tension and shear loading

For combined tension and shear loading see section "Anchor Design".

Precalculated values

Design resistance calculated according ETAG 001, Annex C and data given in ETA-02/0042, issue 2008-01-10. All data applies to concrete C 20/25 – $f_{ck,cube} = 25 \text{ N/mm}^2$. HSL-3-SK and HSL-3-SH is only available up to M12.

Recommended loads can be calculated by dividing the design resistance by an overall partial safety factor for action $\gamma = 1,4$. The partial safety factors for action depend on the type of loading and shall be taken from national regulations.

Design resistance

Single anchor, no edge effects

| Anchor size | | M8 | M10 | M12 | M16 | M20 |
|---|------------------------------------|------|------|------|------|------|
| Min. base material thickness h_{min} [mm] | | 120 | 140 | 160 | 200 | 250 |
| | Tensile N_{Rd} | | | | | |
| | HSL-GR | [kN] | 13,0 | 16,4 | 20,1 | 28,1 |
| | Shear V_{Rd} , without lever arm | | | | | |
| | HSL-GR | [kN] | 16,0 | 25,6 | 35,3 | 51,3 |

Single anchor, min. edge distance ($c = c_{min}$)

| Anchor size | | M8 | M10 | M12 | M16 | M20 |
|---|------------------------------------|------|-----|------|------|------|
| Min. base material thickness h_{min} [mm] | | 120 | 140 | 160 | 200 | 250 |
| Min. edge distance c_{min} [mm] | | 60 | 70 | 80 | 100 | 125 |
| | Tensile N_{Rd} | | | | | |
| | HSL-GR | [kN] | 7,8 | 10,1 | 12,6 | 18,4 |
| | Shear V_{Rd} , without lever arm | | | | | |
| | HSL-GR | [kN] | 6,4 | 8,4 | 10,6 | 15,5 |

Double anchor, no edge effects, min. spacing ($s = s_{min}$), (load values are valid for one anchor)

| Anchor size | | M8 | M10 | M12 | M16 | M20 |
|---|------------------------------------|------|------|------|------|------|
| Min. base material thickness h_{min} [mm] | | 120 | 140 | 160 | 200 | 250 |
| Min. spacing s_{min} [mm] | | 100 | 160 | 240 | 240 | 300 |
| | Tensile N_{Rd} | | | | | |
| | HSL-GR | [kN] | 8,9 | 12,6 | 17,3 | 22,9 |
| | Shear V_{Rd} , without lever arm | | | | | |
| | HSL-GR | [kN] | 16,0 | 25,6 | 35,3 | 51,3 |